

Benchmark Example No. 20

## Passive Earth Pressure I

SOFiSTiK | 2020

**VERiFiCATION**  
**BE20 Passive Earth Pressure I**

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The manual and the program have been thoroughly checked for errors. However, SOFiSTiK does not claim that either one is completely error free. Errors and omissions are corrected as soon as they are detected.

The user of the program is solely responsible for the applications. We strongly encourage the user to test the correctness of all calculations at least by random sampling.

**Front Cover**

Project: Queensferry Crossing | Photo: Bastian Kratzke

## Overview

<b>Element Type(s):</b>	C2D
<b>Analysis Type(s):</b>	STAT, MNL
<b>Procedure(s):</b>	LSTP
<b>Topic(s):</b>	SOIL
<b>Module(s):</b>	TALPA
<b>Input file(s):</b>	<a href="#">passive_earth_pressure.dat</a>

## 1 Problem Description

The problem consists of a soil mass retained by a wall as shown in Fig. 1. The horizontal passive earth pressure is determined and is compared to the value obtained for the case of the soil mass externally forced to its limiting strength.

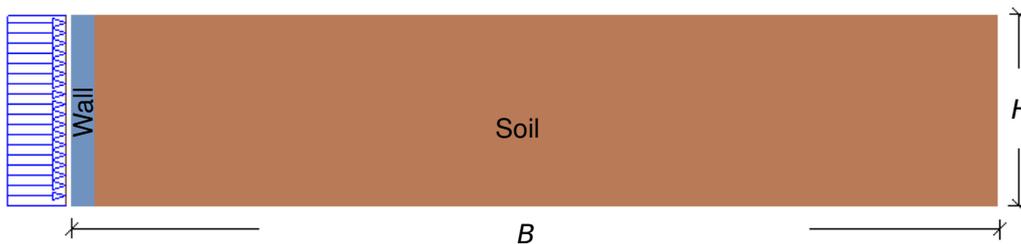


Figure 1: Problem Description

## 2 Reference Solution

When a retaining wall is forced against a soil mass, lateral passive earth pressure is exerted from the soil to the wall. In order to describe the horizontal component of the pressure the soil will exert, an earth pressure coefficient  $K_{ph}$  according to Coulomb theory is used:

$$K_{ph} = \frac{\cos^2(\phi - \alpha)}{\left(1 - \sqrt{\frac{\sin(\phi + \delta_p) \cdot \sin(\phi + \beta)}{\cos(\alpha + \delta_p) \cdot \cos(\alpha + \beta)}}\right)^2 \cos^2 \alpha}, \quad (1)$$

where the parameters  $\alpha$ ,  $\phi$ ,  $\delta_p$  and  $\beta$  are defined in Fig. 2. The wall friction angle is denoted by  $\delta_p$  and the soil friction angle by  $\phi$ . The horizontal passive earth pressure resultant is [1]:

$$E_{ph} = \frac{1}{2} \gamma H^2 K_{ph}. \quad (2)$$

In order to account for the development of irreversible strains in the soil, under the action of the passive load, a plasticity model has to be utilised. Whether plasticity occurs in a calculation, can be evaluated with a yield function  $f$ , where the condition  $f = 0$  stands for the plastic yielding. This condition can be represented as a surface in principal stress space. In this Benchmark, the Mohr-Coulomb model is adopted, which represents an elastic perfectly-plastic behaviour. A perfectly-plastic model corresponds to a fixed yield surface, i.e. a yield surface that is fully defined by model parameters and is not affected by

plastic straining. Moreover, for stress state within the yield surface, the behaviour is purely elastic and all strains are reversible. Hence, the Mohr-Coulomb model requires the input of a total of five parameters, the Young's modulus  $E$  and Poisson's ratio  $\nu$  for the definition of the elasticity, and three for the plasticity, the friction angle  $\phi$ , the cohesion  $c$  and the dilatancy angle  $\psi$ . The dilatancy angle is involved in the plastic potential function and controls the evolution of plastic volumetric strain increments [2].

$$f = \sigma_1 - \frac{1 - \sin\phi}{1 + \sin\phi} \cdot \sigma_3 - \frac{2c \cos\phi}{1 + \sin\phi}, \quad (3)$$

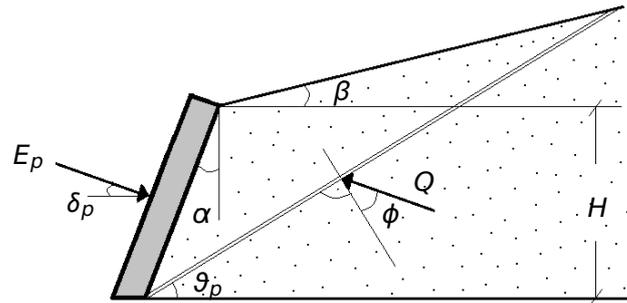


Figure 2: Passive Earth Pressure by Coulomb

The yield function for the Mohr-Coulomb model [2] is defined by Eq. 3, where  $\sigma_1$  and  $\sigma_3$  are the principal stresses, and its yield surface is shown in Fig. 3.

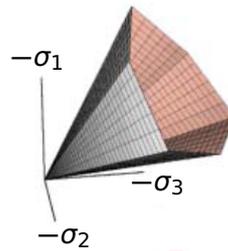


Figure 3: Mohr-Coulomb Yield Surface in Principal Stress Space

### 3 Model and Results

The properties of the model are defined in Table 1. The Mohr-Coulomb plasticity model is used for the modelling of the soil behaviour. The load is defined as a unit support displacement in the  $x$ -direction and is increased gradually until a limit value. It is applied at node 405, which is kinematically coupled with the wall nodes as shown in Fig. 4, and therefore corresponds to a uniformly applied load at the wall nodes. Maximum displacement is recorded for each loading increment, and the curve of horizontal passive earth pressure-displacement (Fig. 5) is plotted against the reference solution according to Coulomb theory.

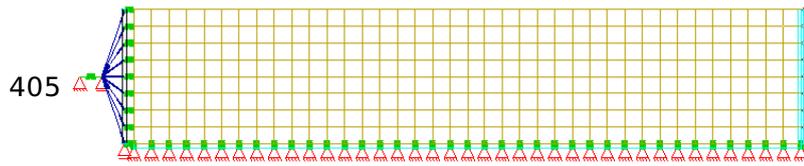


Figure 4: Finite Element Model

Table 1: Model Properties

Material Properties		Geometric Properties		Loading
Wall	Soil	Wall	Soil	
$E = 30000 \text{ MPa}$	$E = 300 \text{ MPa}$	$B = 0.1 \text{ m}$	$B = 30 \text{ m}$	$W_x = 1 \text{ mm}$
$\nu = 0.18$	$\nu = 0.20$	$H = 0.8 \text{ m}$	$H = 6 \text{ m}$	
$\gamma = 24 \text{ kN/m}^3$	$\gamma = 19 \text{ kN/m}^3$			
	$c = 1 \text{ kN/m}^2$			
	$\phi = 38^\circ$			
	$\psi = 6^\circ$			
	$\delta_p = \phi/3$			

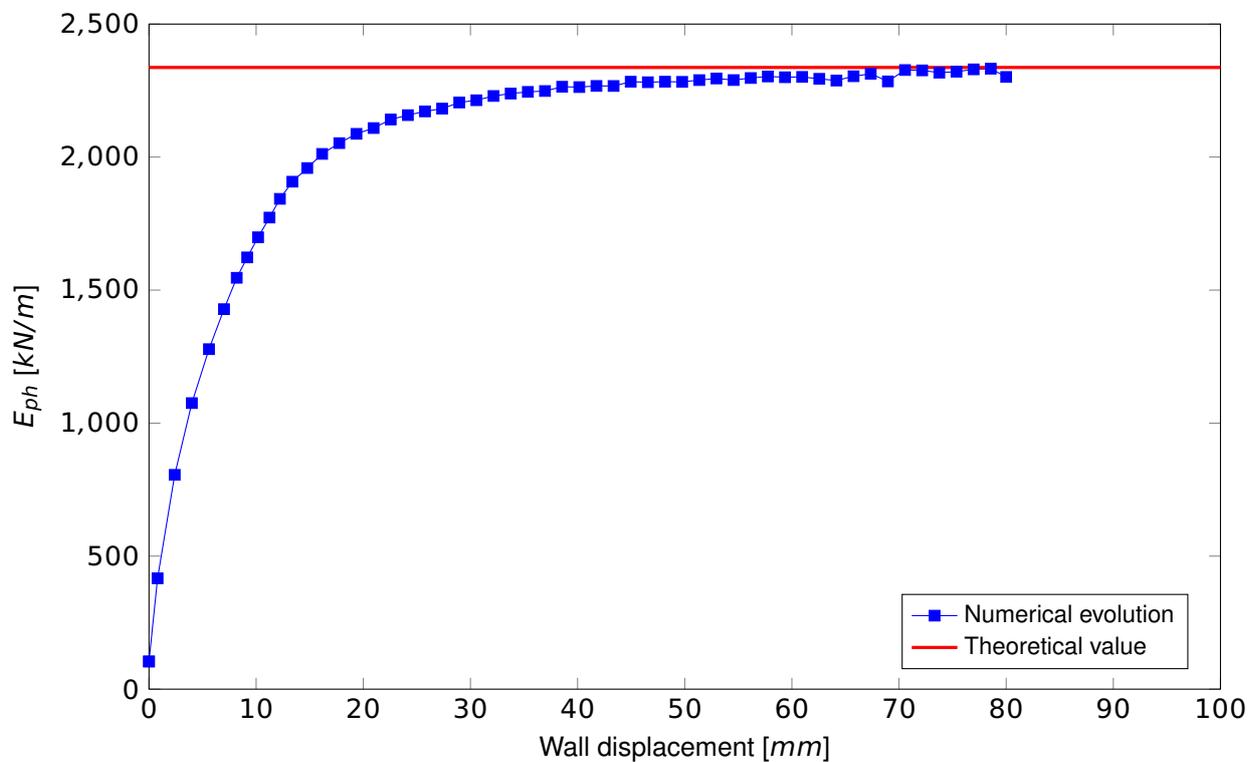


Figure 5: Horizontal Passive Earth Pressure-Displacement Curve

## 4 Conclusion

This example examines the horizontal passive earth pressure determination for a soil mass retained by a wall. The Mohr-Coulomb model for the definition of the soil material behaviour is adopted. It has been shown that the behaviour of the soil is captured accurately.

## 5 Literature

- [1] K. Holschemacher. *Entwurfs- und Berechnungstabeln für Bauingenieure*. 3rd. Bauwerk, 2007.
  - [2] *AQUA Manual: Materials and Cross Sections*. Version 18-0. SOFiSTiK AG. Oberschleißheim, Germany, 2017.
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